

# Worst-case design of plane frames using order statistics

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### Background



Background

#### **Deterministic design**

#### **Uncertainty in real world**



## Suppose uncertainty exists in Young's modulus

**Source**: A. Asadpoure *et al*, Robust topology optimization of structures with uncertainties in stiffness - Application to truss structures, 2011, Computers and Structures.

**Considering Uncertainty** 



### **Existing Challenges**

#### (1) Unknown distribution of uncertainty

#### Two kinds of probability-based design





Minimize:  $E(f(X;U)) + k\sigma(f(X;U))$ 



-5

-10

0

5

10

-10

-5

0

### **Existing Challenges**

#### (2) Exact value of worst-case event

#### Handle the uncertainty with worst-case design





Hard to obtain the exact worst structural response even if Θ is simple



**Existing Challenges** 

#### (3) Melting nodes



#### Desirable to avoid existence of extremely short member

**Existing Challenge** 

#### (4) Stability constraint

**Optimal solution with slender elements** 

**Different global** constraints (b) vol = 0.1440 (c) vol = 0.1597 (d) vol = 0.1582 (e) vol = 0.1574י∳ <sup>F</sup> (f) vol = 0.1567 (g) vol = 0.1562 (h) vol = 0.1558 (i) vol = 0.1552

**Source**: AG. Weldeyesus *et al*, Truss geometry and topology optimization with global stability constraints, 2020, Structural and Multidisciplinary Optimization

### **Optimization with global stability**

$$\begin{array}{l} \text{Minimize } \sigma = \max_{\substack{i=1,2,\cdots m_{\text{e}} \\ j=1,2,\cdots p}} \left( \sigma_{V,ij} \left( \boldsymbol{x}_{\text{free}}, \boldsymbol{y}_{\text{free}}, \boldsymbol{A} \right) \right) \\ \text{subject to } \frac{1}{\lambda^{\text{cr}}} \left( \boldsymbol{x}_{\text{free}}, \boldsymbol{y}_{\text{free}}, \boldsymbol{A} \right) \leq \frac{1}{\lambda_{\text{L}}}; \quad V \left( \boldsymbol{x}_{\text{free}}, \boldsymbol{y}_{\text{free}}, \boldsymbol{A} \right) \leq V_{\text{U}}; \\ \underline{\boldsymbol{x}}_{\text{free}} \leq \boldsymbol{x}_{\text{free}} \leq \overline{\boldsymbol{x}}_{\text{free}}; \quad \underline{\boldsymbol{y}}_{\text{free}} \leq \boldsymbol{y}_{\text{free}} \leq \overline{\boldsymbol{y}}_{\text{free}}; \quad \underline{\boldsymbol{A}} \leq \boldsymbol{A} \leq \overline{\boldsymbol{A}} \end{array}$$

**Smallest positive eigenvalue:** 

$$\left(\boldsymbol{K}\left(\boldsymbol{x}_{\text{free}},\boldsymbol{y}_{\text{free}},\boldsymbol{A}\right)-\lambda\boldsymbol{K}_{G}\left(\boldsymbol{x}_{\text{free}},\boldsymbol{y}_{\text{free}},\boldsymbol{A}\right)\right)\boldsymbol{\Phi}=0$$

x<sub>free</sub>: x coordinates of free nodes;
y<sub>free</sub>: y coordinates of free nodes;
A : Cross-sectional areas
V : Structural volume

*K* : Elastic stiffness matrix

*K*<sub>G</sub> : Geometry stiffness matrix



### Force density method for shape optimization



**Source**: (left) JY Zhang, M Ohsaki. Adaptive force density method for form-finding problem of tensegrity structures. (Right) MO Ruy *et* al The natural force density method for the shape finding of taut structures

## FDM is widely used in form-finding of tension and tensegrity structure

$$q = \frac{N}{L}$$
**Force density Member length Diagonal matrix**

$$x_{\text{free}} = -(\tilde{Q}_{\text{free}}^T \operatorname{diag}(q) \tilde{Q}_{\text{free}})^{-1} \tilde{Q}_{\text{free}}^T \operatorname{diag}(q) \tilde{Q}_{\text{fix}} x_{\text{fix}}$$

$$y_{\text{free}} = -(\tilde{Q}_{\text{free}}^T \operatorname{diag}(q) \tilde{Q}_{\text{free}})^{-1} \tilde{Q}_{\text{free}}^T \operatorname{diag}(q) \tilde{Q}_{\text{fix}} y_{\text{fix}}$$

**Connectivity matrix** 

### Introducing Force density method



**Frame structure** 

FDM can not be directly applied



### Introducing Force density method



### Uncertainty in member stiffness

#### **Uncertainty in nodal locations**





### Uncertainty in member stiffness

#### **Uncertainty in intermediate nodal locations**



### Uncertainty in member stiffness

#### **Uncertainty in cross-sectional areas**



$$A_i' = A_i + \Delta A_i$$

*A'* for all elements in member *i* 



**Worst-case design with uncertainties**  $\theta = (\Delta x, \Delta y, \Delta A)$ 

100 $\beta$ th (0< $\beta$ <1) quantile of stress  $\sigma^{\max-\beta}$  and  $\gamma^{\max-\beta}$ 

Probability 
$$\left\{ \sigma \left( \boldsymbol{x}_{\text{free}} \left( \boldsymbol{q} \right), \boldsymbol{y}_{\text{free}} \left( \boldsymbol{q} \right), \boldsymbol{A}; \boldsymbol{\theta} \right) \leq \sigma^{\max - \beta} \right\} = \beta$$
  
Probability  $\left\{ \gamma \left( \boldsymbol{x}_{\text{free}} \left( \boldsymbol{q} \right), \boldsymbol{y}_{\text{free}} \left( \boldsymbol{q} \right), \boldsymbol{A}; \boldsymbol{\theta} \right) \leq \gamma^{\max - \beta} \right\} = \beta$ 



Probability 
$$\left\{ \sigma \left( \boldsymbol{x}_{\text{free}} \left( \boldsymbol{q} \right), \boldsymbol{y}_{\text{free}} \left( \boldsymbol{q} \right), \boldsymbol{A}; \boldsymbol{\theta} \right) \leq \sigma^{\max - \beta} \right\} = \beta$$
  
Probability  $\left\{ \gamma \left( \boldsymbol{x}_{\text{free}} \left( \boldsymbol{q} \right), \boldsymbol{y}_{\text{free}} \left( \boldsymbol{q} \right), \boldsymbol{A}; \boldsymbol{\theta} \right) \leq \gamma^{\max - \beta} \right\} = \beta$ 

Given *m* sets of 
$$\theta_1, \theta_2, ..., \theta_m$$
, obtain *m* response  
 $\sigma_1 = \sigma(x_{\text{free}}(q), y_{\text{free}}(q), A; \theta_1), ..., \sigma_m = \sigma(x_{\text{free}}(q), y_{\text{free}}(q), A; \theta_m)$   
 $\gamma_1 = \sigma(x_{\text{free}}(q), y_{\text{free}}(q), A; \theta_1), ..., \gamma_m = \sigma(x_{\text{free}}(q), y_{\text{free}}(q), A; \theta_m)$ 

#### and place them in a descending order

$$\sigma_{1:m} \geq \ldots \geq \sigma_{k:m} \geq \ldots \geq \sigma_{m:m}; \gamma_{1:m} \geq \ldots \geq \gamma_{k:m} \geq \ldots \geq \gamma_{m:m}; 1 \leq k \leq m$$



#### **Based on the statistical inference theory of order statistics**

$$\alpha_{k} = \Pr\left\{F_{\sigma}\left(\sigma_{k:m}^{cr}\right) \ge \beta\right\} = \sum_{r=0}^{m-k} \binom{m}{r} \beta^{r} (1-\beta)^{m-r} \qquad \begin{array}{c} \text{Relation bet}\\ k \text{ and } \beta \end{array}$$
$$\alpha_{k} = \Pr\left\{F_{\gamma}\left(\gamma_{k:m}^{cr}\right) \ge \beta\right\} = \sum_{r=0}^{m-k} \binom{m}{r} \beta^{r} (1-\beta)^{m-r}$$



between

**Relation between** *k* and  $\beta$  ( $\alpha_k = 0.9, m = 200$ )

	_			_	_		-			_
k	1	2	3	4	5	6	7	8	9	10
в	0.989	0.981	0.974	0.967	0.960	0.954	0.948	0.942	0.936	0.930
k	11	12	13	14	15	16	17	18	19	20
в	0.924	0.918	0.912	0.907	0.901	0.895	0.890	0.884	0.878	0.873

$$\begin{array}{l} \text{Minimize } \sigma^{\max} = \max_{\theta \in \Omega} \sigma \left( x_{\text{free}} \left( q \right), y_{\text{free}} \left( q \right), A; \theta \right) \\ \text{subject to } \gamma^{\text{cr,max}} = \max_{\theta \in \Omega} \left( \gamma^{\text{cr}} \left( x_{\text{free}} \left( q \right), y_{\text{free}} \left( q \right), A; \theta \right) \right) \leq \gamma_{\text{U}}; \quad V \left( x_{\text{free}} \left( q \right), y_{\text{free}} \left( q \right), A \right) \leq V_{\text{U}}; \\ \underline{q} \leq q \leq \overline{q}; \quad \underline{A} \leq A \leq \overline{A} \\ \text{is rewritten by using order statistics} \\ \text{Minimize } \sigma_{k:m} \left( x_{\text{free}} \left( q \right), y_{\text{free}} \left( q \right), A; \Theta \right) \\ \text{subject to } \gamma^{\text{cr}}_{k:m} \left( x_{\text{free}} \left( q \right), y_{\text{free}} \left( q \right), A; \Theta \right) \leq \gamma_{\text{U}}; \quad V \left( x_{\text{free}} \left( q \right), y_{\text{free}} \left( q \right), A \right) \leq V_{\text{U}}; \\ \underline{q} \leq q \leq \overline{q}; \quad \underline{A} \leq A \leq \overline{A} \end{array}$$

**Smaller order**  $k \rightarrow$  **Larger robustness level** 

### Penalization method for singularity phenomenon

### **Singularity in stress and stability** Large element stress **Small cross-sectional area Small linear buckling** load **Penalization method for small** A $\hat{\sigma}_{V,i} = \left(A_i / \overline{A}\right)^{\eta} \max_{i=1,2...n} \left(\sigma_{V,ij}\right) \qquad \hat{K}_{G,i} = \left(A_i / \overline{A}\right)^{\rho} \times K_{G,i}$



Fig. 2 Feasible domain under formulation (3)

**Source**: X Guo *et al*, Optimum design of truss topology under buckling constraints, 2005, Structural and Multidisciplinary Optimization

### Numerical Example

#### > Example:



Initial condition:  $F = 200 \text{ kN}; E = 2 \times 10^{11} \text{ Pa}; V_{\text{U}} = 0.02 \text{m}^3$ Sample size m = 150; k = 1;

Design variables:  $A = (A_1, A_2, ..., A_{10}); q = (q_1, q_2, ..., q_{10})$  $1 \times 10^{-7} \text{m}^2 \le A \le 0.05 \text{ m}^2; -1000 \le q \le 1000$ 

Solved by generalized reduced gradient (GRG) method

**Pin support** 

### Numerical Example

#### **Optimal results (at nominal condition)**



**Considering Uncertainty** 



#### Without Uncertainty

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Solution	$\sigma$ (MPa)	$\sigma_{1:150}^{\max}$ (MPa)	$\gamma^{\rm cr}$	$\lambda^{cr}$	$\gamma_{1:150}^{ m cr}$	$\lambda_{1:150}^{cr}$	<i>V</i> (m <sup>3</sup> )
R	103.7015	234.7208	0.0868	11.5117	0.0914	10.9328	0.02
D	84.5490	354.7003	0.2899	3.4490	0.3193	3.1314	0.02

### Numerical Example

#### **Optimal results (at worst case condition)**



#### **Solution R**

**Solution D** 

1000

Solution	$\sigma$ (MPa)	$\sigma_{ m 1:150}^{ m max}~( m MPa)$	$\gamma^{\rm cr}$	$\lambda^{cr}$	$\gamma_{1:150}^{ m cr}$	$\lambda_{1:150}^{cr}$	V (m <sup>3</sup> )
R	103.7015	234.7208	0.0868	11.5117	0.0914	10.9328	0.02
D	84.5490	354.7003	0.2899	3.4490	0.3193	3.1314	0.02

### Conclusions and future work

> The proposed method has the following conclusions :

- The robustness of the objective and constraints are represented by the *k*th order statistics.
- An auxiliary truss to which the FDM is applied is used to define the geometry of the frame.
- The stress and geometrical stiffness matrix of a thin element is penalized with respect to the cross-sectional area.
- Solutions with and without considering uncertainties may have different shapes and topologies.

### Conclusions and future work

#### **>**Future work:

- Since the uncertainty is directly incorporated in the optimization problem, the analytical gradient is hard to derive. Maybe decoupling or approximation technique would be useful for improving its convergence and application on large-scale problem.
- The robustness is determined directly by the order statistics, and maybe summary statistics (such as statistical moments) would be helpful to give the similar information of uncertainty with smaller sample size.



## Thanks for your kind attention



**Reporter : Wei Shen** 

